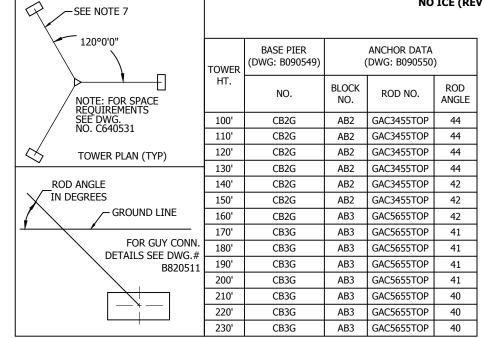


65G TOWER GUYING DETAILS
100' - 230'
90MPH 3-SECOND GUST WIND SPEED
NO ICE (REV G)
70 MPH FASTEST MILE WIND SPEED
NO ICE (REV F)



#### **GENERAL NOTES:**

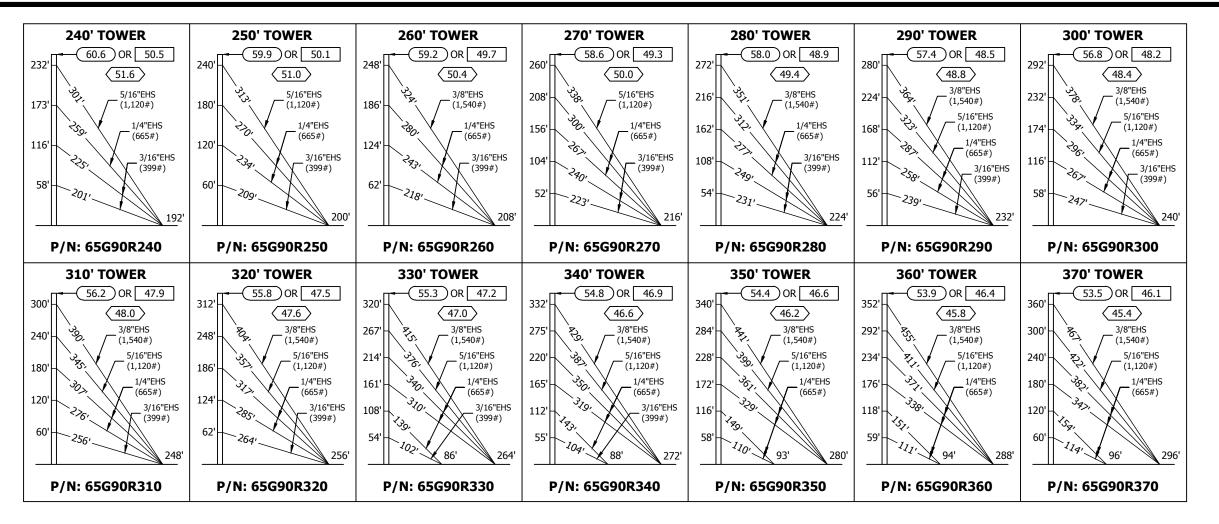
- TOWER DESIGNS ARE IN ACCORDANCE WITH ANSI/TIA-222-F & ANSI/TIA-222-G, CLASS I STRUCTURES.
- 2. —ALLOWABLE PROJ. AREA (SQ. FT.) FOR EXPOSURE B (REV G).

  ALLOWABLE PROJ. AREA (SQ. FT.) FOR EXPOSURE C (REV G).
- ALLOWABLE PROJ. AREA (SQ.FT.) (REV F).
- 3. EFFECTIVE PROJ. AREAS MUST NOT EXCEED THE AREAS SHOWN.
- 4. ANTENNAS AND MOUNTS ARE ASSUMED SYMMETRICALLY PLACED AT THE TOWER TOP.
- 5. DESIGNS ASSUME ONE 1/2" DIA. LINE ON EACH TOWER FACE.
- 6. FOR GUY HARDWARE INSTALLATION DETAILS, SEE DWG. A871382
- 7. ANCHOR RADIUS IS FROM TOWER BASE TO INTERSECTION OF ROD WITH GROUND.
- 8. TOWER DESIGNS AND GUY CHORD LENGTHS SHOWN ARE BASED ON LEVEL GROUND.
  ADD 6 PERCENT TO CHORD LENGTHS (FOR SAG AND CONNECTIONS) FOR FINAL CUT
  LENGTHS. ( ) INDICATES INITIAL TENSION FOR GUY WIRES IN POUNDS AT 60 DEGREES
  FAHRENHEIT.
- DO NOT INSTALL OR DISMANTLE TOWERS WITHIN FALLING DISTANCE OF ELECTRICAL AND/OR TELEPHONE LINES.
- 10. TOWER ERECTION AND DISMANTLING MUST BE DONE BY QUALIFIED AND EXPERIENCED PERSONNEL.
- 11. TEMPORARY STEEL GUYS, WHEN REQUIRED DURING ERECTION OR DISMANTLING, MUST BE SUPPLIED AND INSTALLED BY THE ERECTOR.
- 12. INSTALL WARNING PLATE (P/N: ACWS) IN A HIGHLY VISIBLE LOCATION.
- ALL ANTENNA INSTALLATIONS MUST BE GROUNDED IN ACCORDANCE WITH LOCAL AND NATIONAL CODES.
- 14. EXTRA CABLE CLAMPS HAVE BEEN PROVIDED FOR TURNBUCKLE SAFETY REQUIREMENTS. FOR DETAILS SEE DWG. B680324 LATEST REVISION.
- 15. PURCHASER SHALL VERIFY THE INSTALLATION IS IN CONFORMANCE WITH LOCAL, STATE AND FEDERAL REQUIREMENTS FOR OBSTRUCTION MARKING AND LIGHTING.
- 16. TOLERANCE ON TOWER STEEL IS EQUAL TO PLUS 1% AND MINUS 1/2%.
- 17. DESIGNS ASSUME THAT, AS A MINIMUM, MAINTENANCE AND INSPECTION WILL BE PERFORMED OVER THE LIFE OF THE STRUCTURE IN ACCORDANCE WITH ANSI/TIA/EIA-222-G.
- 18. ANCHOR RODS CORROSION PROTECTION METHODS TO BE PROVIDED BY OTHERS.
- 19. SECTION 65G (10'), WHEN USED, WILL BE LOCATED AT TOP OF TOWER.

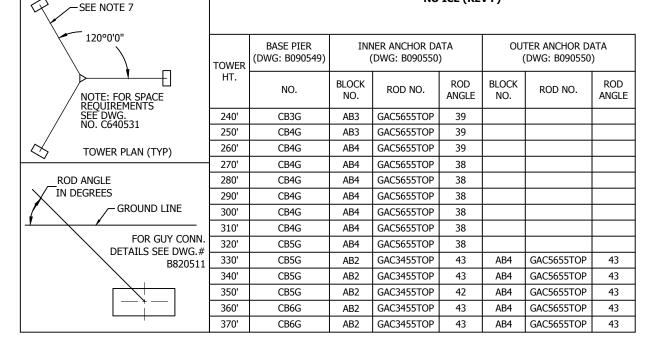
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# 65G TOWER GUYING DETAILS 240' - 370' 90MPH 3-SECOND GUST WIND SPEED NO ICE (REV G) 70 MPH FASTEST MILE WIND SPEED NO ICE (REV F)



#### **GENERAL NOTES:**

- 1. TOWER DESIGNS ARE IN ACCORDANCE WITH ANSI/TIA-222-F & ANSI/TIA-222-G, CLASS I STRUCTURES.
- 2. —ALLOWABLE PROJ. AREA (SQ. FT.) FOR EXPOSURE B (REV G).

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- ALLOWABLE PROJ. AREA (SQ.FT.) (REV F).
- 3. EFFECTIVE PROJ. AREAS MUST NOT EXCEED THE AREAS SHOWN.
- 4. ANTENNAS AND MOUNTS ARE ASSUMED SYMMETRICALLY PLACED AT THE TOWER TOP. 5. DESIGNS ASSUME ONE 1/2" DIA. LINE ON EACH TOWER FACE.
- 6. FOR GUY HARDWARE INSTALLATION DETAILS, SEE DWG. A871382
- 7. ANCHOR RADIUS IS FROM TOWER BASE TO INTERSECTION OF ROD WITH GROUND.
- 8. TOWER DESIGNS AND GUY CHORD LENGTHS SHOWN ARE BASED ON LEVEL GROUND. ADD 6 PERCENT TO CHORD LENGTHS (FOR SAG AND CONNECTIONS) FOR FINAL CUT LENGTHS. ( ) INDICATES INITIAL TENSION FOR GUY WIRES IN POUNDS AT 60 DEGREES FAHRENHEIT.
- DO NOT INSTALL OR DISMANTLE TOWERS WITHIN FALLING DISTANCE OF ELECTRICAL AND/OR TELEPHONE LINES.
- 10. TOWER ERECTION AND DISMANTLING MUST BE DONE BY QUALIFIED AND EXPERIENCED PERSONNEL.
- 11. TEMPORARY STEEL GUYS, WHEN REQUIRED DURING ERECTION OR DISMANTLING, MUST BE SUPPLIED AND INSTALLED BY THE ERECTOR.
- 12. INSTALL WARNING PLATE (P/N: ACWS) IN A HIGHLY VISIBLE LOCATION.
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- 19. SECTION 65G (10'), WHEN USED, WILL BE LOCATED AT TOP OF TOWER.

REVISIONS REV. DESCRIPTION DWN CHK APP REVISED ANCHOR ROD ANGLES & ADDED ANCHOR DATA DRAWING NUMBERS DATE: Feb/19/2010 REMOVED "ANCHOR ROD SLOPE" NOTE 2 DATE: May/05/2010  DATE: May/05/2010  DWS JDM HA  DWG REFERENCE	FILE	NO.	٠			
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SECTION ASSEMBLY 65G 90MPH 3-SECOND GUST NO ICE

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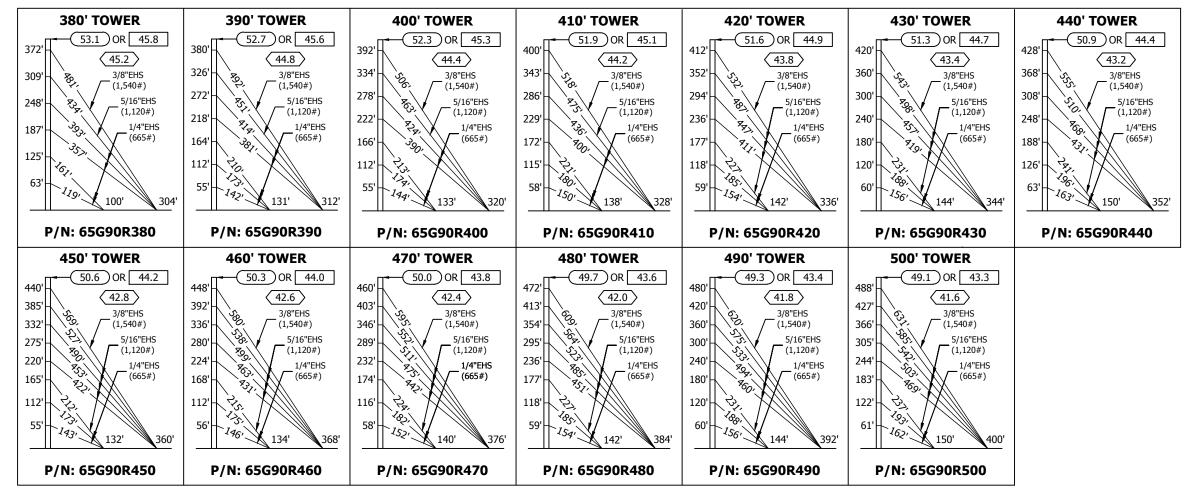
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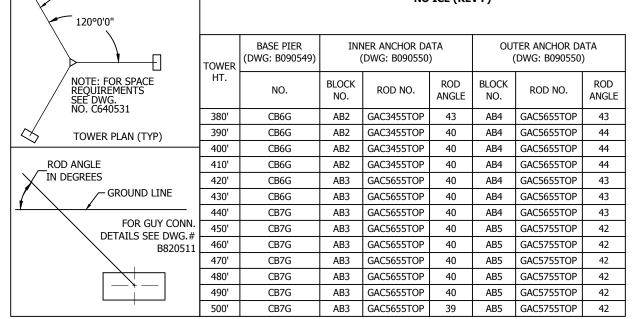
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-SEE NOTE 7



65G TOWER GUYING DETAILS
380' - 500'
90MPH 3-SECOND GUST WIND SPEED
NO ICE (REV G)
70 MPH FASTEST MILE WIND SPEED
NO ICE (REV F)



#### **GENERAL NOTES:**

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- 19. SECTION 65G (10'), WHEN USED, WILL BE LOCATED AT TOP OF TOWER.

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SECTION ASSEMBLY 65G 90MPH 3-SECOND GUST NO ICE

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# FOUNDATION AND ANCHOR TOLERANCES ALL FOUNDATIONS

- 1. CONCRETE DIMENSIONS PLUS OR MINUS 1" (25mm).
- 2. DEPTH OF FOUNDATION PLUS 3" (76mm) OR MINUS 0".
- 3. DRILLED FOUNDATIONS OUT OF PLUMB 1.0 DEGREE.
- 4. REINFORCING STEEL PLACEMENT PER A.C.I. 301.
- 5. PROJECTION OF EMBEDMENTS PLUS OR MINUS 1/8" (3mm).
- 6. VERTICAL EMBEDMENTS OUT OF PLUMB 0.5 DEGREE.

#### **ANCHOR BOLTS**

- 7. MAXIMUM DISTANCE FROM CENTERLINE OF ANCHOR BOLTS TO CENTERLINE OF FOUNDATION 1/24 OF PIER DIAMETER UP TO A MAXIMUM OF 2" (51mm).
- 8. ANCHOR BOLT SPACING 1/16" (2mm).
- 9. ANCHOR BOLT CIRCLE ORIENTATION 0.25 DEGREE.
- 10. ANCHOR BOLT CIRCLE DIAMETER PLUS OR MINUS 1/16" (2mm).

### **SELF-SUPPORTING TOWERS**

- 11. FACE SPREAD DIMENSION CENTER TO CENTER OF ANCHOR BOLT CIRCLES PLUS OR MINUS 1/16" (2mm) OR 1/16" (2mm) PER 20 FT. (6m) OF FACE SPREAD.
- 12. MAXIMUM DIFFERENCE BETWEEN ANY TWO FOUNDATION ELEVATIONS 1/2" (13mm).

#### **GUYED TOWERS**

- 13. GUY RADIUS PLUS OR MINUS 5% OF DISTANCE SPECIFIED.
- 14. ANCHOR ELEVATION PLUS OR MINUS 5% OF GUY RADIUS.
- 15. ANCHOR ALIGNMENT (PERPENDICULAR TO GUY RADIUS) 1.0 DEGREE.
- 16. ANCHOR ROD SLOPE PLUS OR MINUS 1.0 DEGREE.
- 17. ANCHOR ROD ALIGNMENT WITH GUY RADIUS PLUS OR MINUS 1.0 DEGREE.
- 18. ANCHOR HEAD OUT OF PLUMB 1.0 DEGREE.
- 19. GUY INITIAL TENSION PLUS OR MINUS 10% OF TENSION SPECIFIED.

NOTE: TOLERANCES IN NOTES 13 AND 14 CAN NOT OCCUR SIMULTANEOUSLY

## WARNING!!!

AFTER ANCHOR BOLTS ARE INSTALLED IN CONCRETE HAS TAKEN ITS INITIAL SET, ANCHOR BOLTS MUST NOT BE MOVED, BENT OR REALIGNED IN ANY MANNER. A NUT LOCKING DEVICE MUST BE INSTALLED ON ALL ANCHOR BOLTS.

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# STANDARD FOUNDATION NOTES ANSI/TIA-222-G

 STANDARD FOUNDATION DESIGNS ARE IN ACCORDANCE WITH ANSI/TIA-222-G, "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES", SECTION 9 AND ANNEX F FOR THE FOLLOWING PRESUMPTIVE CLAY SOIL PARAMETERS:

N (blows/ft)	Ф (deg)	Y (lb/ft3)	C (psf)	Ultimate E (psf [kPa	)	Ultimate Skin Friction (psf)	k (pci)	<b>E</b> 50
[blows/m]	(ueg)	[kN/m3]	[kPa]	Shallow Fnds.	Deep Fnds.	[kPa]	[kN/m3]	
8 [26]	0	110 [17]	1000 [48]	5000 [240]	9000 [431]	500 [24]	150 [41,000]	0.01

- 2. THE PURCHASER MUST VERIFY THAT ACTUAL SITE SOIL PARAMETERS MEET OR EXCEED ANSI/TIA-222-G PRESUMPTIVE CLAY SOIL DESIGN PARAMETERS AND THAT THE PENETRATION AND/OR ZONE OF SEASONAL MOISTURE VARIATION AT THE SITE. FOUNDATION DESIGN MODIFICATIONS MAY BE REQUIRED IN THE EVENT PRESUMPTIVE CLAY SOIL PARAMETERS ARE NOT APPLICABLE FOR THE ACTUAL SUBSURFACE CONDITIONS ENCOUNTERED.
- 3. A SITE-SPECIFIC INVESTIGATION IS REQUIRED FOR CLASS III STRUCTURES IN ACCORDANCE WITH ANSI/TIA-222-G.
- 4. FOUNDATION DESIGNS ASSUME FIELD INSPECTIONS WILL BE PERFORMED BY THE PURCHASER'S REPRESENTATIVE TO VERIFY THAT CONSTRUCTION MATERIALS, INSTALLATION METHODS AND ASSUMED DESIGN PARAMETERS ARE ACCEPTABLE BASED ON THE CONDITIONS EXISTING AT THE SITE.
- 5. WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES, SAFETY REGULATIONS AND UNLESS OTHERWISE NOTED, THE LATEST REVISION OF ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". PROCEDURES FOR THE PROTECTION OF EXCAVATIONS, EXISTING CONSTRUCTION AND UTILITIES SHALL BE ESTABLISHED PRIOR TO FOUNDATION INSTALLATION.
- 6. CONCRETE MATERIALS SHALL CONFORM TO THE APPROPRIATE STATE REQUIREMENTS FOR EXPOSED STRUCTURAL CONCRETE.
- 7. PROPORTIONS OF CONCRETE MATERIALS SHALL BE SUITABLE FOR THE INSTALLATION METHOD UTILIZED AND SHALL RESULT IN DURABLE CONCRETE FOR RESISTANCE TO LOCAL ANTICIPATED AGGRESSIVE ACTIONS. THE DURABILITY REQUIREMENT OF ACI 318 CHAPTER 4 SHALL BE SATISFIED BASED ON THE CONDITIONS EXPECTED AT THE SITE. AS A MINIMUM, CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI (27.6 MPa) IN 28 DAYS.
- 8. MAXIMUM SIZE OF AGGREGATE SHALL NOT EXCEED SIZE SUITABLE FOR INSTALLATION METHOD UTILIZED OR 1/3 CLEAR DISTANCE BEHIND OR BETWEEN REINFORCING. MAXIMUM SIZE MAY BE INCREASED TO 2/3 CLEAR DISTANCE PROVIDED WORKABILITY AND METHODS OF CONSOLIDATION SUCH AS VIBRATING WILL PREVENT HONEYCOMBS OR VOIDS.
- REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO THE REQUIREMENTS OF ASTM A615
  GRADE 60 UNLESS OTHERWISE NOTED. SPLICES IN REINFORCEMENT SHALL NOT BE ALLOWED
  UNLESS OTHERWISE INDICATED.
- 10. REINFORCING CAGES SHALL BE BRACED TO RETAIN PROPER DIMENSIONS DURING HANDLING, THROUGHOUT PLACEMENT OF CONCRETE AND DURING EXTRACTION OF TEMPORARY CASING.
- 11. WELDING IS PROHIBITED ON REINFORCING STEEL AND EMBEDMENTS.

- 12. MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE 3 INCHES (76 mm) UNLESS
  OTHERWISE NOTED. APPROVED SPACERS SHALL BE USED TO INSURE A 3 INCH (76 mm) MINIMUM
  COVER ON REINFORCEMENT. CONCRETE COVER FROM TOP OF FOUNDATION TO ENDS OF
  VERTICAL REINFORCEMENT SHALL NOT EXCEED 3 INCHES (76 mm) NOR BE LESS THAN 2 INCHES
  (51 mm).
- 13. SPACERS SHALL BE ATTACHED INTERMITTENTLY THROUGHOUT THE ENTIRE LENGTH OF VERTICAL REINFORCING CAGES TO INSURE CONCENTRIC PLACEMENT OF CAGES IN EXCAVATIONS.
- 14. FOUNDATION DESIGNS ASSUME STRUCTURAL BACKFILL TO BE COMPACTED IN 8 INCH (200 mm)

  MAXIMUM LAYERS TO 95% OF MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT IN

  ACCORDANCE WITH ASTM D698. ADDITIONALLY, STRUCTURAL BACKFILL MUST HAVE A MINIMUM

  COMPACTED UNIT WEIGHT OF 100 POUNDS PER CUBIC FOOT (16 kN/m3).
- 15. FOUNDATION DESIGNS ASSUME LEVEL GRADE AT THE SITE.
- 16. FOUNDATION INSTALLATION SHALL BE SUPERVISED BY PERSONNEL KNOWLEDGEABLE AND EXPERIENCED WITH THE PROPOSED FOUNDATION TYPE. CONSTRUCTION SHALL BE IN ACCORDANCE WITH GENERALLY ACCEPTED INSTALLATION PRACTICES.
- 17. FOR FOUNDATION AND ANCHOR TOLERANCES SEE DRAWING A810214.
- 18. LOOSE MATERIAL SHALL BE REMOVED FROM BOTTOM OF EXCAVATION PRIOR TO CONCRETE PLACEMENT. SIDES OF EXCAVATION SHALL BE ROUGH AND FREE OF LOOSE CUTTINGS.
- 19. CONCRETE SHALL BE PLACED IN A MANNER THAT WILL PREVENT SEGREGATION OF CONCRETE MATERIALS, INFILTRATION OF WATER OR SOIL AND OTHER OCCURRENCES WHICH MAY DECREASE THE STRENGTH OR DURABILITY OF THE FOUNDATION.
- 20. FREE FALL CONCRETE MAY BE USED PROVIDED FALL IS VERTICAL DOWN WITHOUT HITTING SIDES OF EXCAVATION, FORMWORK, REINFORCING BARS, FORM TIES, CAGE BRACING OR OTHER OBSTRUCTIONS. UNDER NO CIRCUMSTANCES SHALL CONCRETE FALL THROUGH WATER.
- 21. CONCRETE SHALL BE PLACED AGAINST UNDISTURBED SOIL EXCEPT FOR PIERS OR PIER AND PAD FOUNDATIONS. FORMS FOR PIERS SHALL BE REMOVED PRIOR TO PLACING STRUCTURAL BACKFILL.
- 22. CONSTRUCTION JOINTS, IF REQUIRED IN PIER MUST BE AT LEAST 12 INCHES (305 mm) BELOW BOTTOM OF EMBEDMENTS AND MUST BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF 1/4 INCH (6 mm). FOUNDATION DESIGN ASSUMES NO OTHER CONSTRUCTION JOINTS.
- 23. CASING, IF USED, SHALL NOT BE LEFT IN PLACE. EQUIPMENT, PROCEDURES, AND PROPORTIONS
  OF CONCRETE MATERIALS SHALL INSURE CONCRETE WILL NOT BE ADVERSELY DISTURBED UPON
  CASING REMOVAL. DRILLING FLUID, IF USED, SHALL BE FULLY DISPLACED BY CONCRETE AND
  SHALL NOT BE DETRIMENTAL TO CONCRETE OR SURROUNDING SOIL. CONTAMINATED CONCRETE
  SHALL BE REMOVED FROM TOP OF FOUNDATION AND REPLACED WITH FRESH CONCRETE.
- 24. TOP OF FOUNDATION SHALL BE SLOPED TO DRAIN WITH A FLOATED FINISHED. EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" X 3/4" (19 mm X 19 mm) MINIMUM.
- 25. FOR ANCHOR BLOCK TYPE FOUNDATIONS, FOR GUYED TOWERS, ADDITIONAL CORROSION PROTECTION MAY BE REQUIRED FOR STEEL GUY ANCHORS IN DIRECT CONTACT WITH SOIL.

  DESIGN ASSUMES PERIODIC INSPECTIONS WILL BE PERFORMED OVER THE LIFE OF THE STRUCTURE TO DETERMINE IF ADDITIONAL ANCHOR CORROSION PROTECTION MEASURES MUST BE IMPLEMENTED BASED ON OBSERVED SITE-SPECIFIC CONDITIONS.

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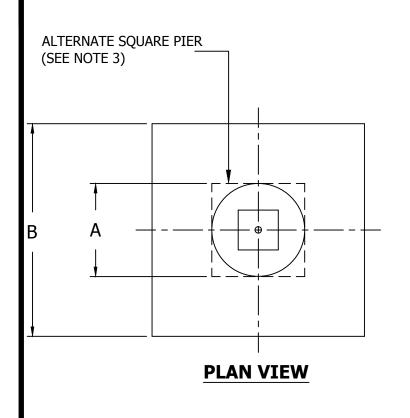
ANSI/TIA-222-G STANDARD FOUNDATION NOTES

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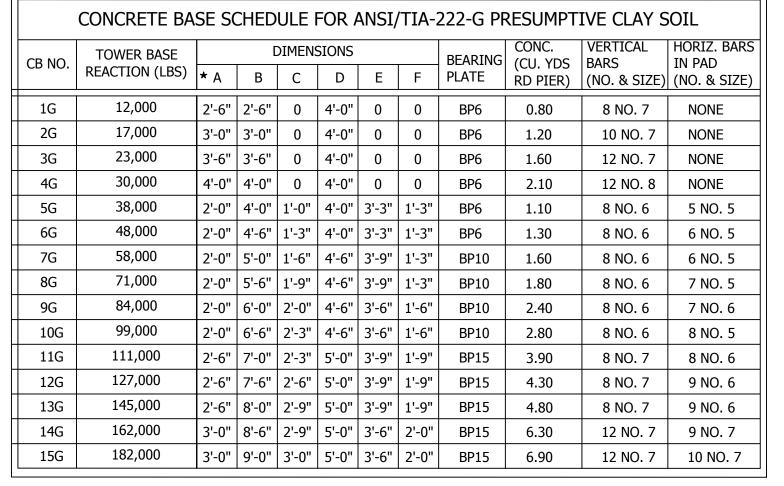
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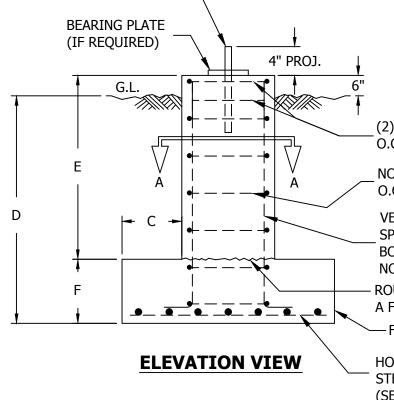
PIER PIN -



VERTICAL BARS EQUALLY SPACED (SEE CHART FOR NO. & SIZE)

ALTERNATE SQUARE PIER
\_ #4 CIRCULAR TIES 3" ON CENTERS
W/ 24" LAPS

SECTION A-A



(2) NO. 4 CIRCULAR TIES @ 2-1/2" O.C. W/ 2" COVER W/ 24" LAPS.

NO. 4 CIRCULAR TIES AT 3" MAX. O.C. W/ 24" LAPS.

VERTICAL RE-BARS EQUALLY SPACED W/ 90° HOOKS AT BOTTOM (SEE CHART FOR NO. & SIZE)

ROUGHEN CONSTRUCTION JOINT TO A FULL AMPLITUDE OF 1/4"

-FOUNDATION PAD

HORIZONTAL REINFORCING STEEL EQUALLY SPACED EACH WAY (SEE CHART FOR NO. & SIZE)

#### NOTES:

- 1. SEE TOWER ASSEMBLY DRAWING FOR FOUNDATION LAYOUT AND PART NUMBERS FOR BEARING PLATE & PIER PIN.
- 2. SEE DRAWING NUMBER B090548 FOR STANDARD FOUNDATION NOTES.
- ★3. USE MIN. 2'-6" SQ. OR 3'-0" DIA. ROUND PIER WHEN BPC45G OR BPC55G IS USED.
- 4. VERTICAL REINFORCING STEEL SHALL BE REPLACED WITH STRAIGHT BARS WHEN NO PAD IS REQUIRED.
- 5. HORIZ. BARS IN CHART REFER ONLY TO THE BARS IN THE FOUNDATION PAD.

KYHN	ĺ
PRODUCT!	ς

DWG REFERENCE

STDPUBLIC
REVISIONS
DESCRIPTION

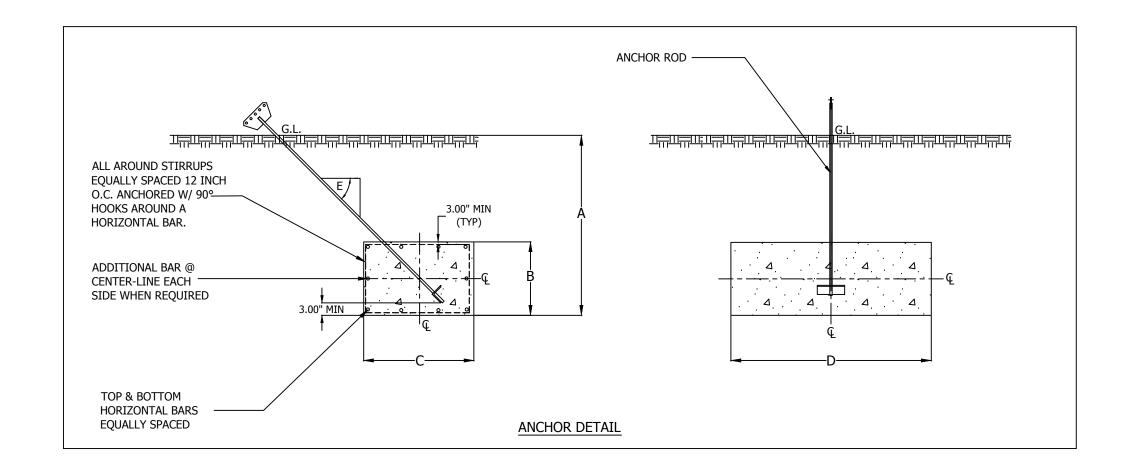
DWN CHK APP

6718 WEST PLANK ROAD PEORIA, IL 61604 TOLL FREE 800-727-ROHN

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FOUNDATION BASE PIER FOR REV. G PRESUMPTIVE CLAY

/Erection/



			CONCRE	TE ANCH	OR BLOCK DATA FOR ANSI/TIA-222	2-G PRESUMPTIV	E CLAY SOIL		
BLOCK	A	ANCHOR DIM	1ENSIONS (I	N.)	HORIZONTAL BARS	STIRRUPS SIZE &	CONCRETE VOL.	UPLIFT	LATERAL
BLOCK	Α	В	С	D	QTY./SIZE	SPACING	(CU. YDS.)	CAPACITY(LBS)	CAPACITY(LBS)
AB1	3'-0"	1'-0"	3'-0"	4'-0"	(8) #5 BARS TOTAL (4) #5 BARS TOP AND BOTTOM LAYERS (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	0.44 PER BLOCK 1.3 TOTAL FOR 3	4,800	2,150
AB2	4'-0"	1'-6"	4'-0"	6'-0"	(10) #6 BARS TOTAL (5) #6 BARS TOP AND BOTTOM LAYERS (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	1.33 PER BLOCK 4.0 TOTAL FOR 3	12,600	6,480
AB3	6'-0"	1'-6"	3'-0"	6'-0"	(8) #6 BARS TOTAL (4) #6 BARS TOP AND BOTTOM LAYERS (0) ADDITIONAL BAR EACH SIDE	#3 @ 12" O.C.	1.0 PER BLOCK 3.0 TOTAL FOR 3	18,700	10,500
AB4	6'-0"	1'-6"	4'-0"	9'-0"	(10) #6 BARS TOTAL (5) #6 BARS TOP AND BOTTOM LAYERS (0) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.0 PER BLOCK 6.0 TOTAL FOR 3	32,500	15,800
AB5	8'-0"	2'-0"	3'-0"	10'-0"	(10) #7 BARS TOTAL (4) #7 BARS TOP AND BOTTOM LAYERS (1) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.22 PER BLOCK 6.7 TOTAL FOR 3	43,000	21,000
AB6	8'-0"	2'-0"	4'-0"	10'-0"	(12) #7 BARS TOTAL (5) #7 BARS TOP AND BOTTOM LAYERS (1) ADDITIONAL BAR EACH SIDE	#4 @ 12" O.C.	2.96 PER BLOCK 8.9 TOTAL FOR 3	52,000	26,500

#### **GENERAL NOTES**

- 1. SEE DRAWING NUMBER B090548 FOR STANDARD FOUNDATION NOTES.
- 2. ALL HORIZONTAL BARS MUST BE CONTINUOUS.
- 3. DUE TO VARIABLES INVOLVED DURING INSTALLATION, IT SHALL BE THE CUSTOMER'S OR INSTALLER'S RESPONSIBILITY TO PROVIDE STRUCTURALLY ADEQUATE SUPPORTS FOR BASE AND ANCHOR CONNECTIONS. IT MAY ALSO BE NECESSARY FOR THE CUSTOMER OR INSTALLER TO SECURE THE SERVICE OF A LOCAL ENGINEER TO DETERMINE THAT INSTALLATION COMPLIES WITH LOCAL BUILDING CODES.
- 4. ADDITIONAL CORROSION PROTECTION MAY BE REQUIRED FOR STEEL GUY ANCHORS IN DIRECT CONTACT WITH SOIL.

ILE	NO.			
	STDPUBLIC			
	REVISIONS			
EV.	DESCRIPTION	DWN	CHK	ΑP
1	AB6 ADDED	FAD	НА	НА
	DATE: Dec/21/2009			
2	SLOPE 'E' NOTES CHANGED	fdm	НА	НА
	DATE: Jan/21/2010			
3	UPDATED LAYOUT	FAD	НА	НА
	DATE: Jul/23/2010			

DWG REFERENCE	



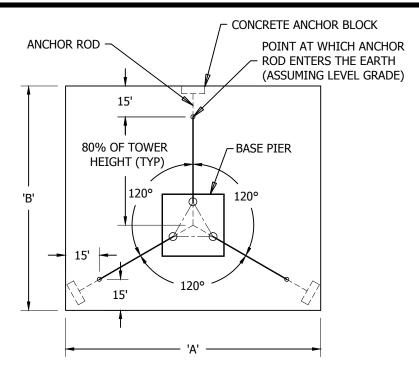
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FOUNDATION ANCHOR BLOCK REV. G PRESUMPTIVE CLAY

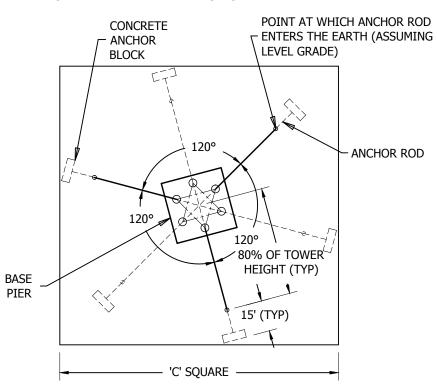
DWN:	FAD	CHK'D:	НА	DATE: Nov/2	24/2009
ENG'R:		łΑ			
	Г	IA			
DRAWIN	IG NO:				REV:
B090550			)		3

(SEE TOWER ASSEMBLY DRAWING FOR ANCHOR ROD SLOPE 'E'.)



#### LAYOUT A

THIS IS THE MINIMUM AREA OF LAND REQUIRED. HOWEVER, THIS AREA WILL NOT ALWAYS PERMIT ORIENTING TOWER INTO THE BEST POSITION FOR ANTENNA PATH DIRECTION.



#### LAYOUT B

THIS IS THE MINUMUM AREA OF LAND REQUIRED TO PERMIT ORIENTING THE TOWER IN ANY POSITION FOR ANTENA PATH DIRECTION.

TOWER	LAYOUT A			LAYOUT B		
HEIGHT	ACRES	Α	В	ACRES	С	
20'	0.08	60'	55'	0.10	65'	
30'	0.12	75'	70'	0.15	80'	
40'	0.17	90'	80'	0.21	95'	
50'	0.21	100'	90'	0.28	110'	
60'	0.28	115'	105'	0.39	130'	
70'	0.35	130'	115'	0.48	145'	
80'	0.43	145'	130'	0.59	160'	
90'	0.50	155'	140'	0.70	175'	
100'	0.59	170'	150'	0.83	190'	
110'	0.70	185'	165'	1.01	210'	
120'	0.80	200'	175'	1.16	225'	
130'	0.94	215'	190'	1.32	240'	
140'	1.04	225'	200'	1.49	255'	
150'	1.16	240'	210'	1.67	270'	
160'	1.32	255'	225'	1.93	290'	
170'	1.46	270'	235'	2.14	305'	
180'	1.64	285'	250'	2.35	320'	
190'	1.76	295'	260'	2.58	335'	
200'	1.92	310'	270'	2.81	350'	
210'	2.13	325'	285'	3.14	370'	
220'	2.31	340'	295'	3.40	385'	
230'	2.50	350'	310'	3.67	400'	
240'	2.68	365'	320'	3.95	415'	
250'	2.88	380'	330'	4.24	430'	
260'	3.13	395'	345'	4.65	450'	
270'	3.34	410'	355'	4.96	465'	
280'	3.57	420'	370'	5.29	480'	
290'	3.80	435'	380'	5.63	495'	
300'	4.03	450'	390'	5.97	510'	
310'	4.33	465'	405'	6.45	530'	
320'	4.53	475'	415'	6.82	545'	
330'	4.84	490'	430'	7.20	560'	
340'	5.10	505'	440'	7.59	575'	
350'	5.37	520'	450'	8.00	590'	
360'	5.71	535'	465'	8.54	610'	
370'	5.94	545'	475'	8.97	625'	
380'	6.30	560'	490'	9.40	640'	
390'	6.60	575'	500'	9.85	655'	
400'	6.91	590'	510'	10.31	670'	
410'	7.23	600'	525'	10.93	690'	
420'	7.55	615'	535'	11.41	705'	
430'	7.96	630'	550'	11.90	720'	
440'	8.29	645'	560'	12.40	735'	
450'	8.64	660'	570'	12.91	750'	
460'	9.00	670'	585'	13.61	770'	
470'	9.36	685'	595'	14.15	785'	
480'	9.80	700'	610'	14.69	800'	
490'	10.18	715'	620'	15.25	815'	
500'	10.49	725'	630'	15.81	830'	

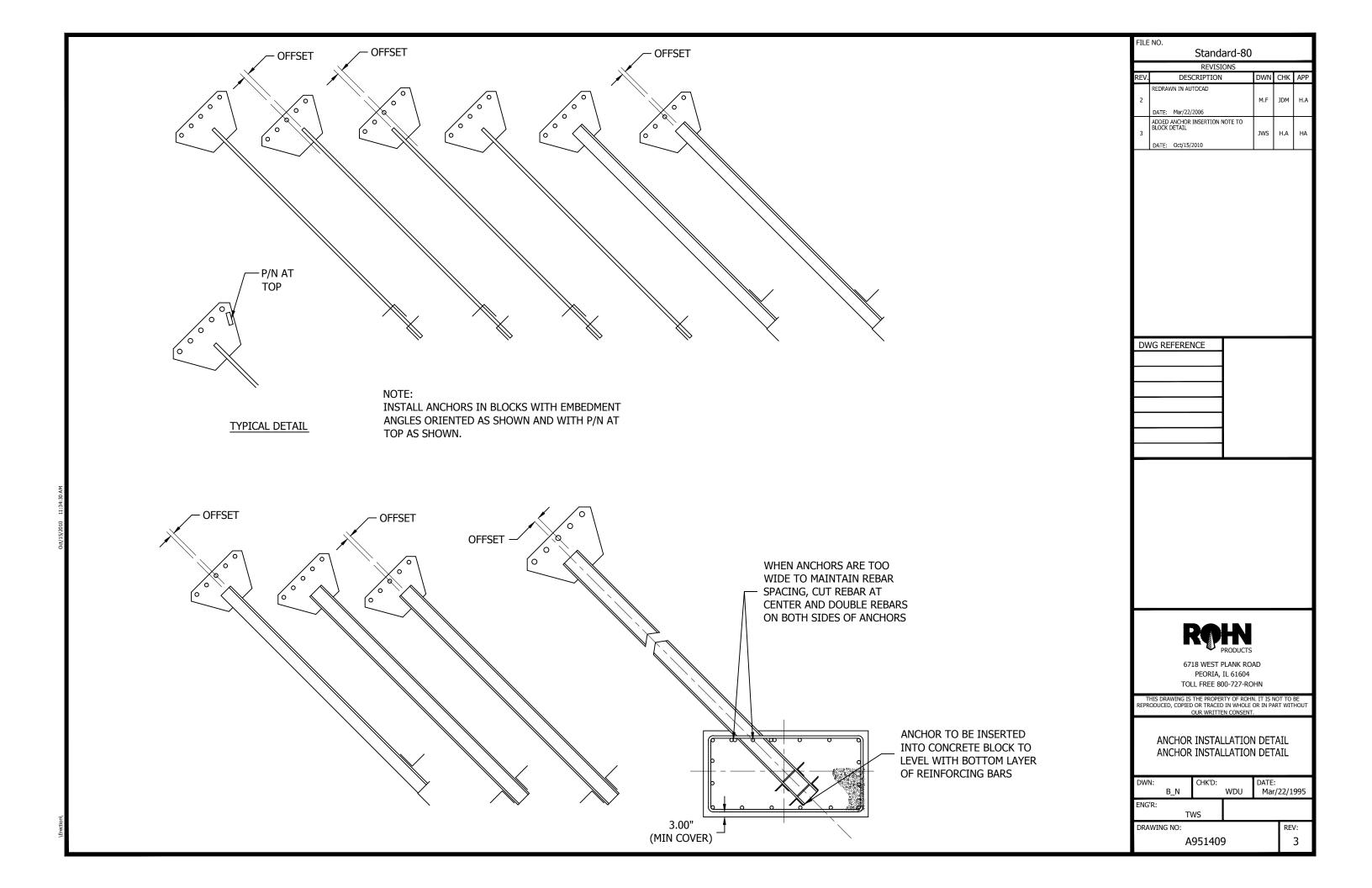
]	TOWER	L	AYOUT A	4	LAYO	UT B
]	HEIGHT	ACRES	Α	В	ACRES	С
	550' 600' 650' 700' 750' 800' 850' 900' 950' 1000' 1150' 1150'	12.59 14.89 17.39 19.97 22.85 25.91 29.17 32.62 36.26 40.10 43.98 48.19 52.60 57.20	795' 865' 935' 1000' 1070' 1140' 1210' 1280' 1450' 1455' 1625' 1695'	690' 750' 810' 870' 930' 990' 1050' 1110' 1170' 1230' 1350' 1410' 1470'	19.01 22.50 26.28 30.36 34.73 39.40 44.35 49.61 55.15 61.00 67.13 73.56 80.28 87.30	910' 990' 1070' 1150' 1230' 1310' 1390' 1470' 1550' 1630' 1710' 1790' 1870' 1950'

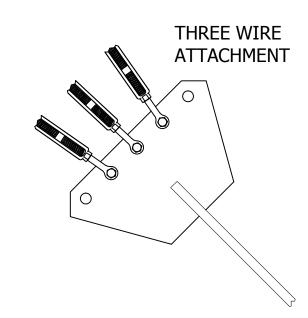
#### **GENERAL NOTES**

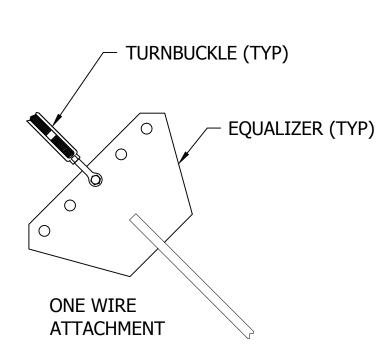
- 1. DUE TO VARIABLES INVOLVED IN ROOF AND OTHER INSTALLATIONS, IT SHALL BE THE RESPONSIBILITY OF THE CUSTOMER OR INSTALLER TO PROVIDE STRUCTURALLY ADEQUATE SUPPORTS FOR PIER AND ANCHOR CONNECTIONS. IT MAY ALSO BE NECESSARY FOR THE CUSTOMER OR INSTALLER TO SECURE THE SERVICE OF A LOCAL ENGINEER TO DETERMINE THAT THE INSTALLATION COMPLIES WITH LOCAL BUILDING CODES.
- 2. FOR RESTRICTED SITES, CUSTOM DESIGNS WITH STRONGER MASTS AND LARGER GUYS MAY BE PROVIDED BY REDUCING THE GUY RADIUS FROM 80% TO 40% OF THE TOWER HEIGHT.

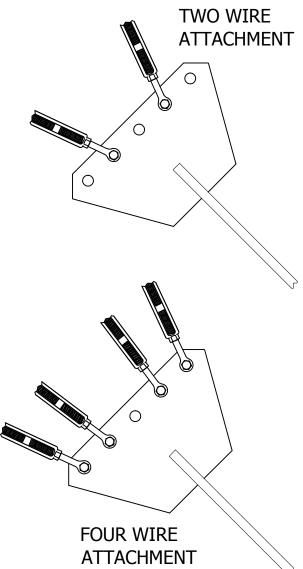
FILE	NO. Standa				
	REVISIO	NS			
REV.	DESCRIPTION DESCRIPTION		DWN	CHK	APP
4	REDRAWN INTO AUTOCAD & ADDED NOTE #2		JDA	JDM	НА
7	DATE: Mar/22/2006		JUA	JUN	11/
DW	/G REFERENCE				
	ROPER DE	RODUCTS	۸D		
	6718 WEST PL PEORIA, IL TOLL FREE 800 HIS DRAWING IS THE PROPERT	- 61604 )-727-RO Y OF ROHN	HN N. IT IS I		
KEPR	ODUCED, COPIED OR TRACED I OUR WRITTEN GUY AN LAND REQUIREI	CHOR			HOUT
DWI	MSR	ОН	DATE Apr	: /15/1	975
F	CW			1	
UKA	WING NO: C640531			RE	
	C640531			1	4

VIV.	CHK D:		DATE:		
MSR		OH	Apr/	15/1975	
G'R:					•
С	W				
RAWING NO:				REV:	
C	640531	L		4	

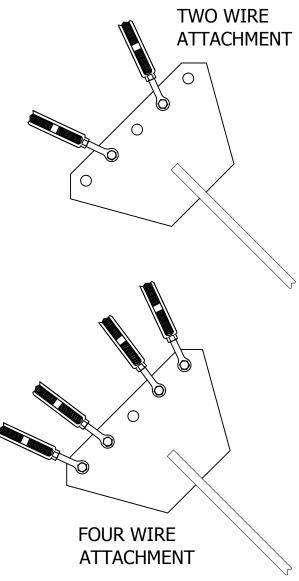








NOTE: SEE TOWER ASSEMBLY DRAWING FOR SIZE AND QTY OF TURNBUCKLES REQUIRED.



Standard-80 DESCRIPTION JDA JDM DWG REFERENCE

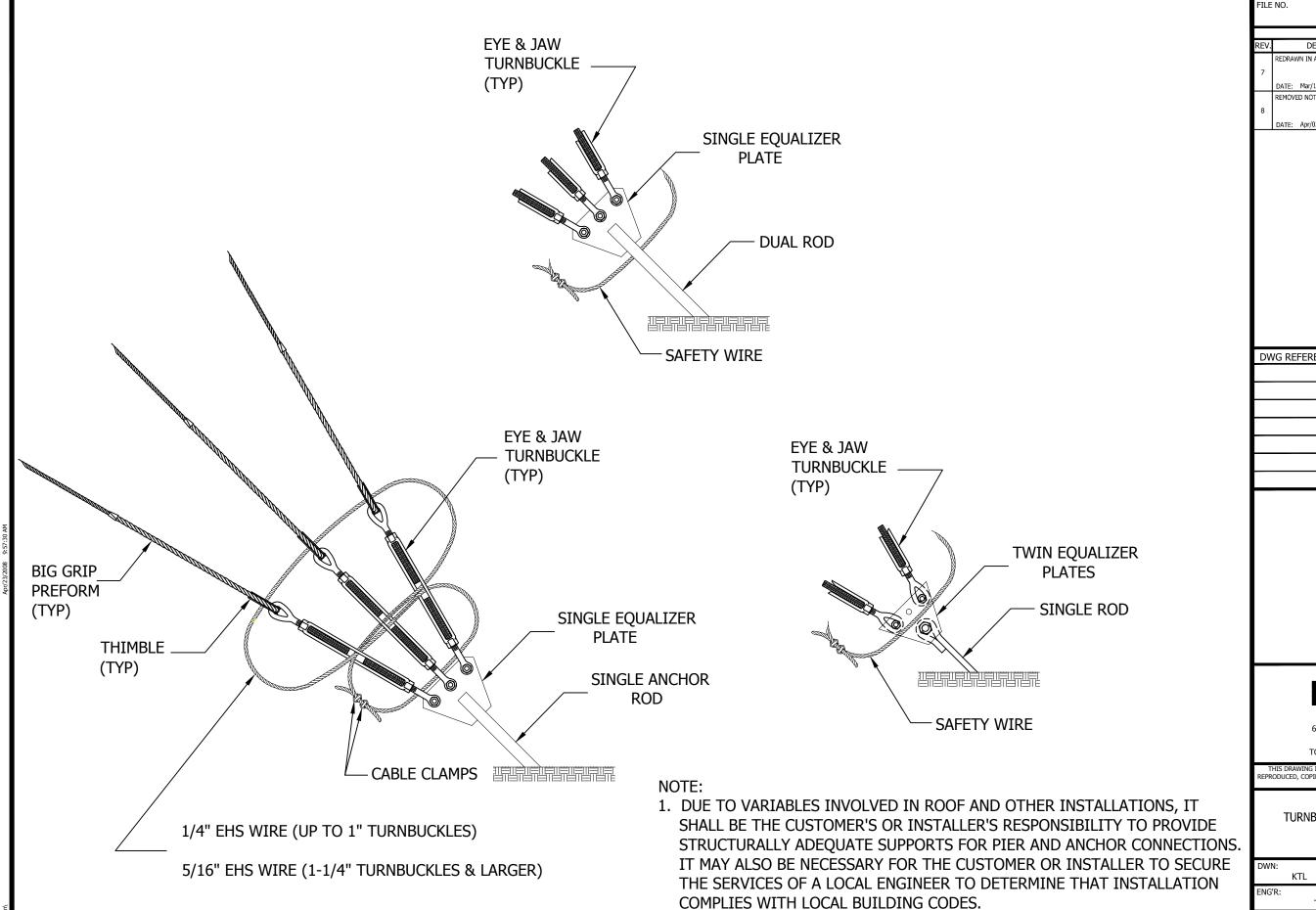
FIVE WIRE

**ATTACHMENT** 

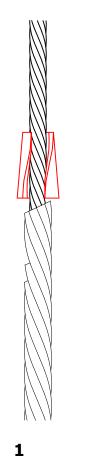
PEORIA, IL 61604 TOLL FREE 800-727-ROHN

ANCHOR ATTACHMENT DETAIL

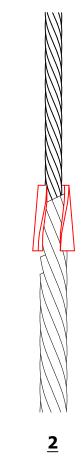
DATE: Jun/23/1982 B820511



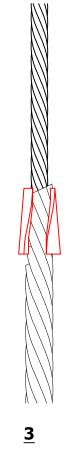
	Standard-8	0		
EV.	REVISIONS DESCRIPTION	DWN	CHK	APP
v .	REDRAWN IN AUTOCAD	DWIN	CHIN	7 11 1
7	DATE: Mar/17/2006	JDM	M.F	H.A
8	REMOVED NOTE #2	JDM	M.F	НА
	DATE: Apr/03/2006			
_				
DW	/G REFERENCE			
	ROHN			
	PRODUCT	rs		
	6718 WEST PLANK R PEORIA, IL 6160			
	TOLL FREE 800-727-F	ROHN		
T REPR	HIS DRAWING IS THE PROPERTY OF RO ODUCED, COPIED OR TRACED IN WHOL OUR WRITTEN CONSEI	E OR IN PA	NOT TO ART WIT	BE HOUT
	CON THE CONSE			
	TURNBUCKLE SAFETY	METH	OD	
ıwd	N: CHK'D:	DATE	:	
ENG	KTL O.H		r/04/1	968
	TWS			
DRA	WING NO: R680324		RE	۷: ه



PLACE THE SLOT SIDE OF THE END SLEEVE OVER THE LONG LEG OF THE DEAD END



DRIVE THE SLEEVE DOWNWARD UNTIL
THE RODS OF THE SHORT LEG ARE
COMPLETELY COVERED



THE RODS OF THE LONG LEG SHOULD BE EVEN WITH, OR MAY EXTEND ABOVE, THE TOP EDGE OF THE SLEEVE

**BE SURE TO SELECT THE PROPER SIZE END SLEEVE** 

FILE	NO.					
		Standa	rd-80			
		REVISIO	NS			
REV.		DESCRIPTION		DWN	CHK	AF
	REDRAWN	IN AUTOCAD				
3				JDM	M.F	H.
	DATE: M	1ar/17/2006				

DWG REFERENCE



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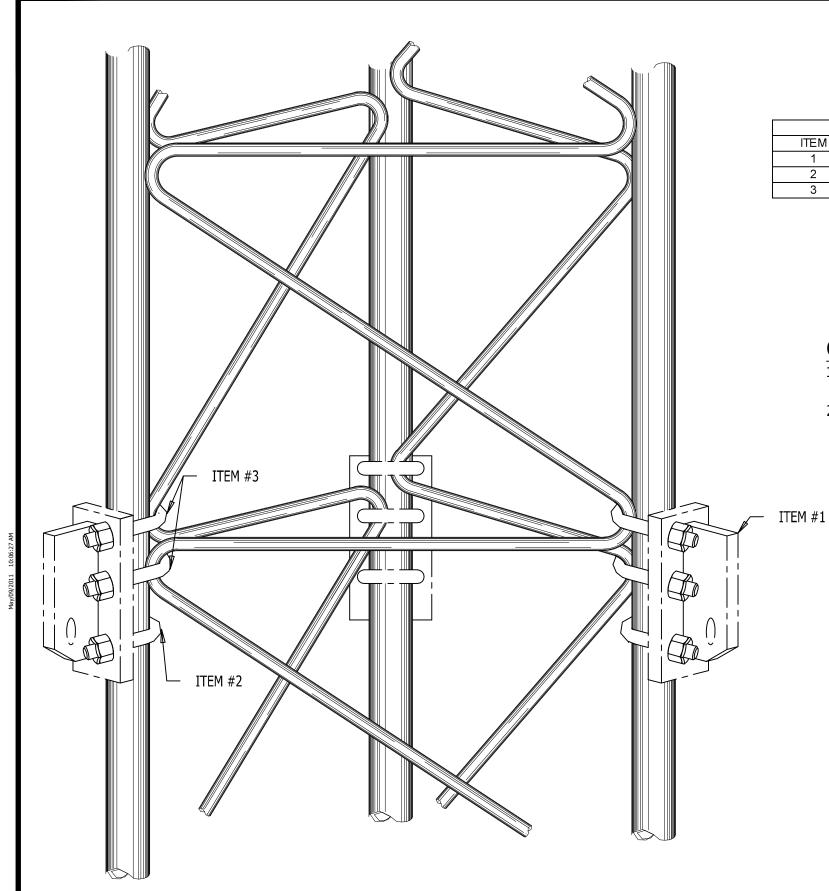
SPLICE CONNECTION FOR BIG GRIP & END SLEEVE

DWN:		CHK'D:		DATE:	
	H.A		RAM	Jun/	09/1970
ENG'R:					
	T۱	NS			
DRAWIN	G NO:				REV:
	В	700607	7		3

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FILE	NO.	Ctand	24d 00			
		REVISI	ard-90			
REV.	DESC	CRIPTION		DWN	CHK	APP
	REDRAWN INTO A					
3				M.F	JDM	НА
	DATE: Apr/04/2					
	REMOVED NOTAT	ION		l l		l
4	DATE: Sep/13/2	2007		J.K	JDM	HA
	DATE: 3ep/13/2	2007				
DW	VG REFEREN	ICE				
			HV			
			PRODUCTS			
				<b>A.D.</b>		
	671	18 WEST I PEORIA,	PLANK ROA IL 61604	4D		
	TOL		01004 00-727-RO	HN		
Т	HIS DRAWING IS	THE PROPER	RTY OF ROHI	N. IT IS N	OT TO	BE
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	\A/T		JY DWARE	νīΤ		
	WIR	C HAKI	JWAKE	VI I		
DWI		CHK'D:	14/ 14	DATE:		00-
	WHW		W.M	Sep	/30/1	987
ENG		OM				
	WING NO:	×1:1			RE	<b>/</b> :
DBV					- INL	
DRA		871382	2			4

DWN:		CHK'D:		DATE:	
WHW	I		W.M	Sep/3	30/1987
ENG'R:					
	RI	MC			
DRAWING NO	:				REV:



	ASSEMBLY P/N GA65GD				
ITEM QTY PART NO. DESCRIPTION DWG NO.					
1	3	B65GD	GUY BRACKET	C870709	
2	3	JR810A	1/2" U-BOLT ASSY	B651028	
3	6	JR65SA	1/2" U-BOLT ASSY	B710909	

# **GENERAL NOTES**

- 1. THE 2 UPPERMOST U-BOLTS MUST INTERCONNECT WITH ZIG-ZAG BRACES AS SHOWN.
- 2. MAXIMUM THIMBLE SIZE =  $\frac{9}{16}$ " HVY.

MAX. REV 'F'	MAX GUY WIRE
VERTICAL PULL	SIZE
5.2 KIPS	3/8" EHS

MAX. REV 'G'	MAX GUY WIRE
VERTICAL PULL	SIZE
9.4 KIPS	3/8" EHS

Standard-65G							
REVISIONS							
REV.	DESCRIPTION	DWN	CHK	APP			
	UPDATED PER ENGINEERING REQUEST						
5		CEJ	JDM	НА			
	DATE: June/14/2011						

DWG REFERENCE

ROHN

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**GUY ASSY 65G** 

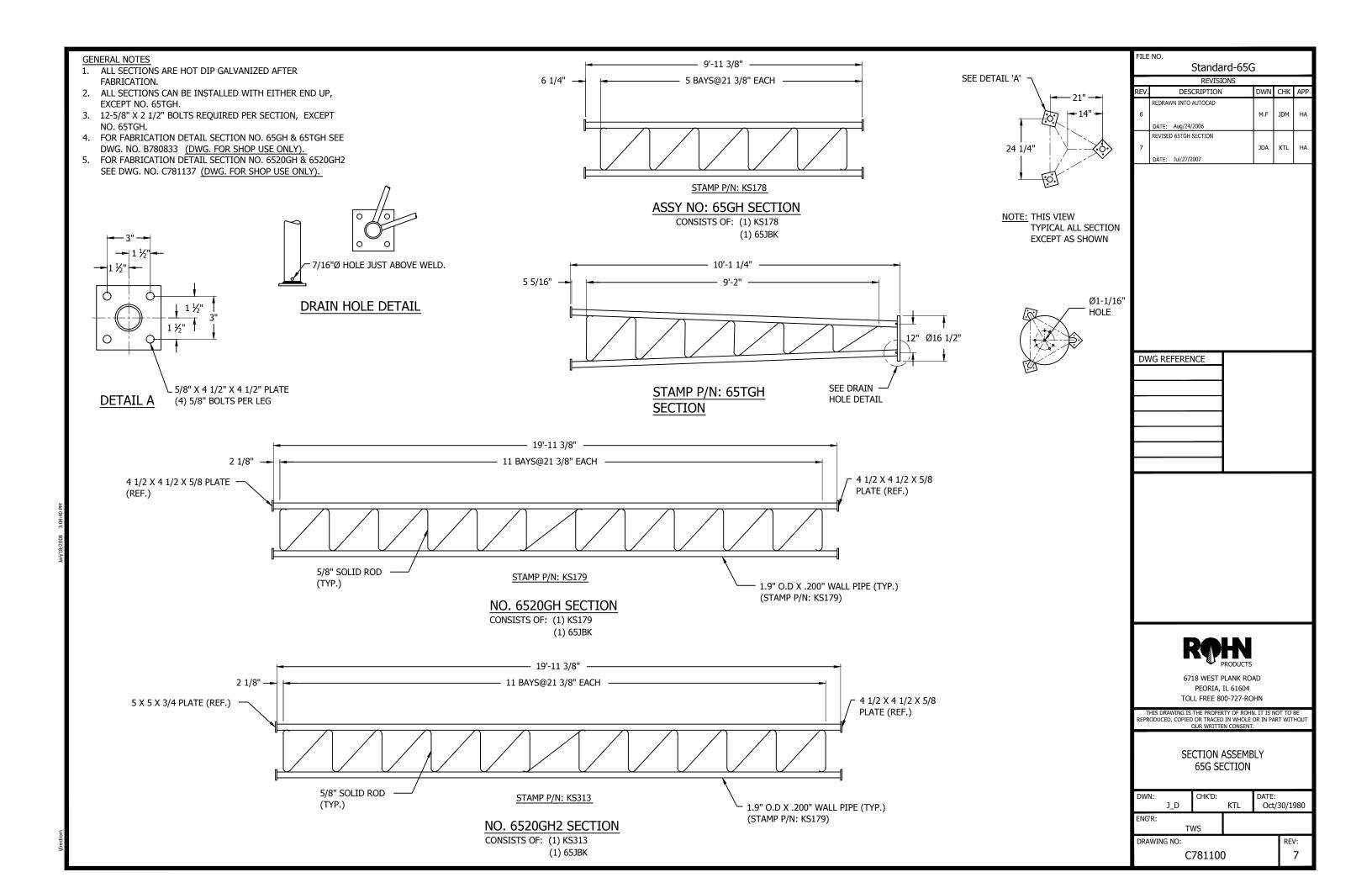
 DWN:
 CHK'D:
 DATE:

 B\_F
 CW
 Sep/22/1987

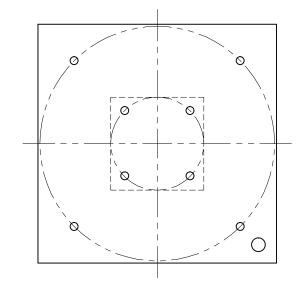
 ENG'R:
 RAM

DRAWING NO:

B870900



ASSEMBLY	BEACON PLATE		CAP PLATE		BOLT ASSEMBLY		
NUMBER	PART NO.	QTY.	PART NO.	QTY.	SIZE	PT.NO.	QTY.
APL1W2WA	APL1W2W	1			3/8X1-1/4	210005GA	6
APL3WNA	APL3WN	1			3/8X1-1/4	210005GA	6
APL4A	APL4	1			1/2X1-1/2	210018GA	4
APL4HA	APL4H	1	CP4H	2	5/8X1-3/4	210030GA	8
APL5A	APL5	1	CP50	2	5/8 X 2	210031GA	8
APL6A	APL6	1	CP60	2	3/4 X 2	210047GA	8
APL7A	APL7	1	CP70	2	7/8 X 3	210062GA	8
APL95A	APL95	1	CP95	2	1 X 3	210067GA	8
APL85A	APL8.5	1			7/8 X 3	210062GA	4
APL78A	APL7.8	1			1 X 3-1/4	210104GA	4
APL788A	APL788	1			3/4 X 3	210051GA	4
APL6A2	APL6	1			3/4X2-3/4	210050GA	4

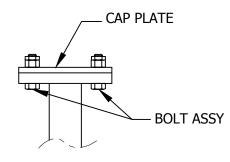


BEACON PLATE INSTALLATION DETAIL PLAN VIEW

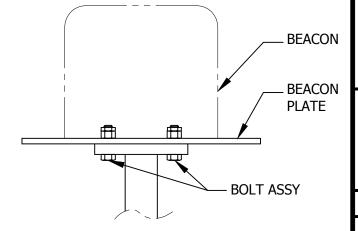
## REFERENCE DRAWINGS

\* ( FAB DWG. FOR SHOP USE ONLY)

- \* 1. FOR BEACON PLATE FABRICATION SEE DWG. NO. SK730369.
- \* 2. FOR CAP PLATE FABRICATION SEE DWG. NO. B760639.
- 3. FOR BOLT ASSEMBLY SEE DWG. NO. C770404.
- \* 4. FOR APL95 BEACON PLATE FABRICATION SEE DWG. NO. B800450.
- ★ 5. FOR APL1W2W FABRICATION SEE DWG. NO. SK740429.
- ★ 6. FOR APL3WN FABRICATION SEE DWG. NO. A730902.



**CAP PLATE INSTALLATION DETAIL** 



BEACON PLATE INSTALLATION DETAIL

ELEVATION VIEW

DWG REFERENCE

RPHN
PRODUCTS

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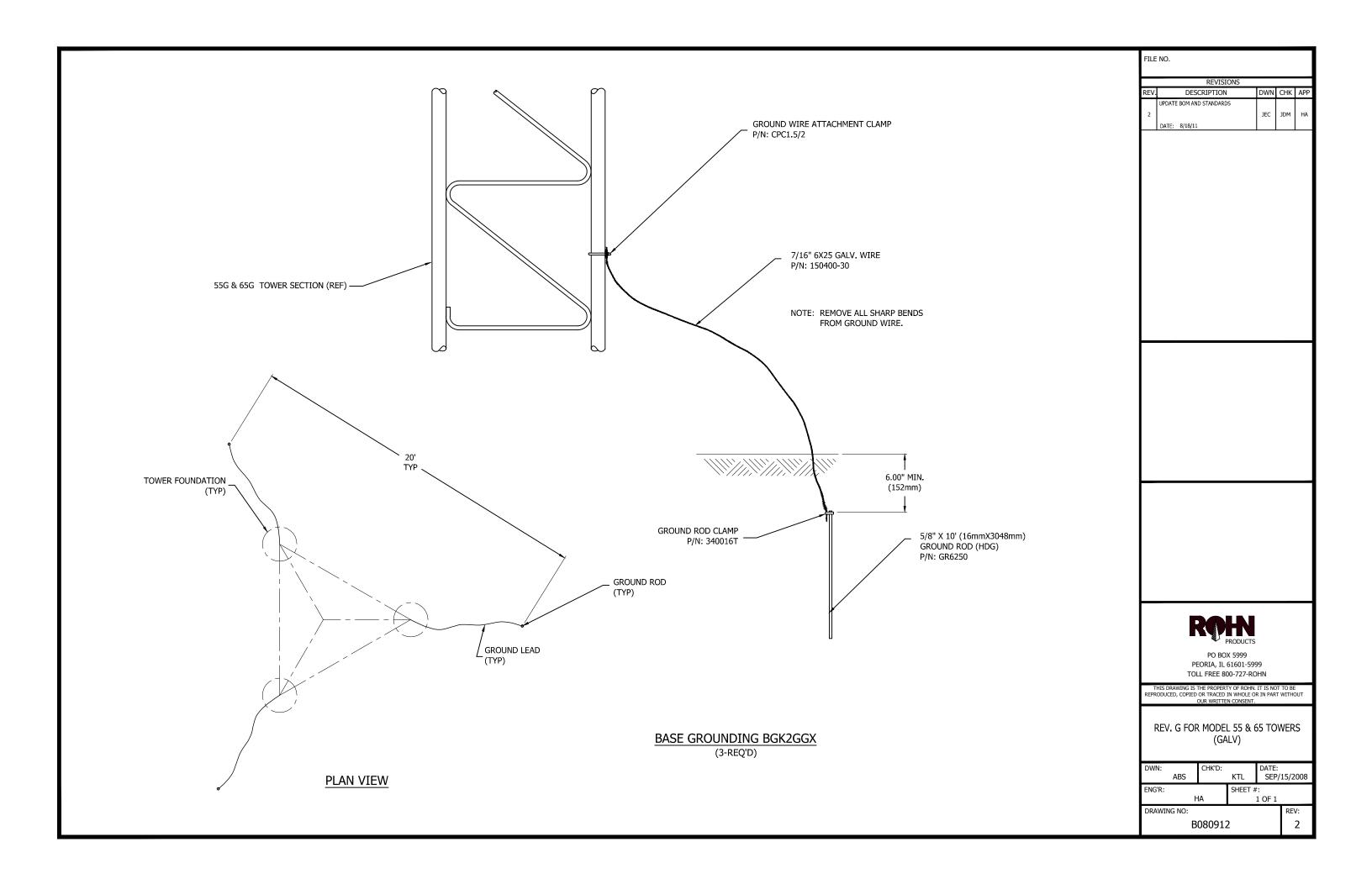
LIGHTING BEACON PLATE ASSY

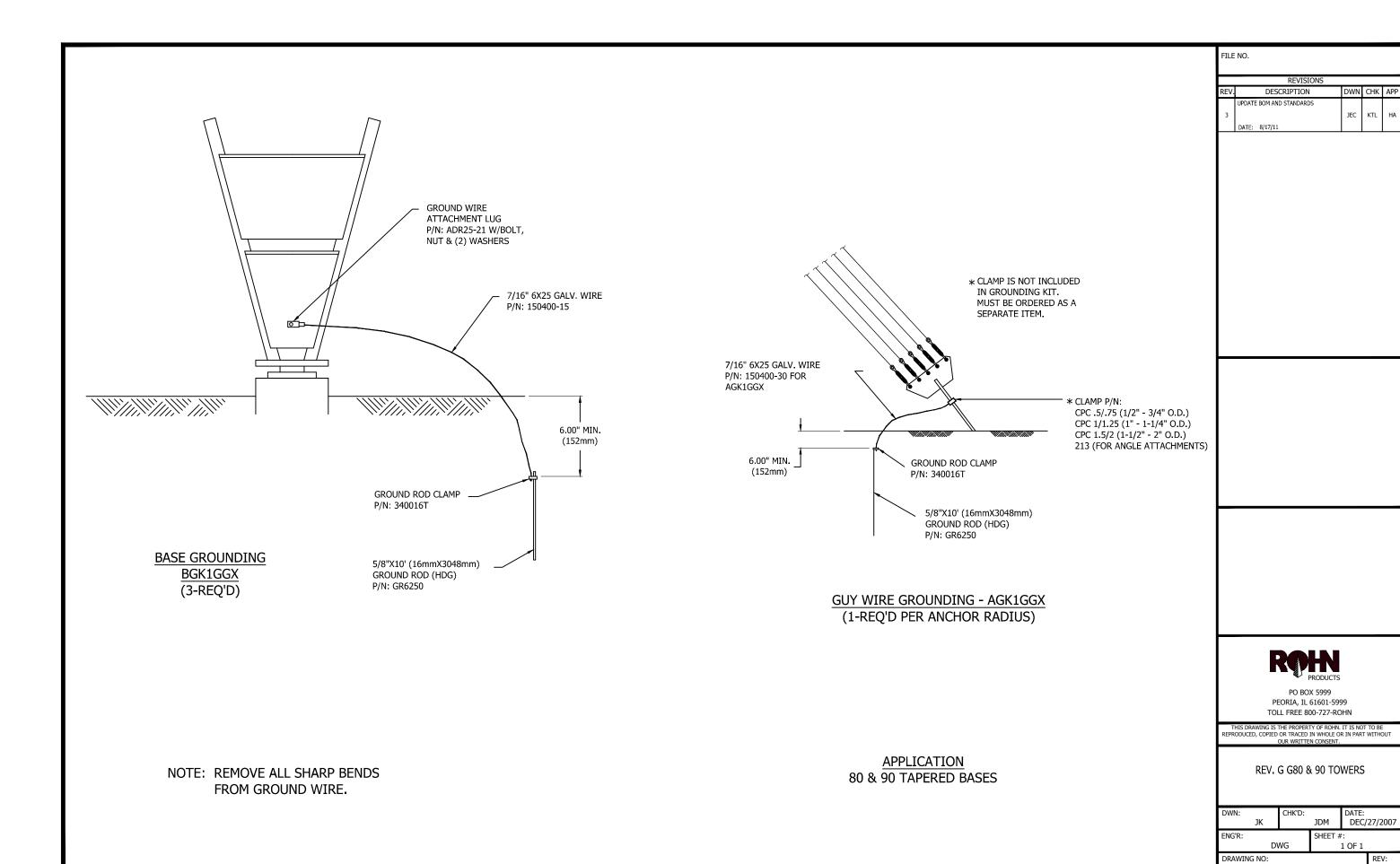
DWN: B\_F CHK'D: KTL Oct/19/1982

ENG'R: TWS

DRAWING NO: REV: B760624 13

=rection\





3

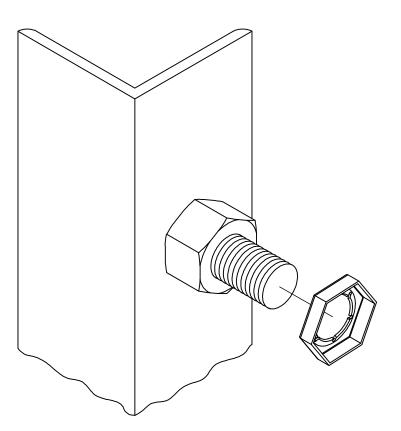
B070996

#### **ASSEMBLY BOLT INSTALLATION**

- 1. UNLESS OTHERWISE SPECIFIED, ASSEMBLY BOLTS AND ANCHOR BOLTS ARE TO BE TIGHTENED TO A SNUG TIGHT CONDITION (MEMBERS IN FIRM CONTACT) AND MUST INCLUDE A NUT LOCKING DEVICE. NO MINIMUM BOLT TENSION OR TORQUE VALUES ARE REQUIRED. WHEN LOCK WASHERS ARE PROVIDED AS A NUT LOCKING DEVICE, REPLACE ANY DAMAGED WASHERS DUE TO OVER TIGHTENING.
- 2. WASHERS ARE TO BE INSTALLED OVER SLOTTED HOLES.

#### **PAL NUT INSTALLATION**

1. PAL NUTS ARE TO BE INSTALLED AFTER NUTS ARE TIGHT AND WITH EDGE LIP OUT (SEE PICTURE). PAL NUTS ARE NOT REQUIRED WHEN SELF-LOCKING NUTS OR LOCK WASHERS ARE PROVIDED.



LILE	NO.			
	REVISIONS			
REV.	DESCRIPTION CHANGE NOTATION.	DWN	CHK	AP
7		JEC	JDM	H/
	DATE: 01/11/12			
_				_
		_		
	ROH	N		
	PROD	JCTS		
	PO BOX 599			
	PEORIA, IL 6160 TOLL FREE 800-72			
Т	HIS DRAWING IS THE PROPERTY OF		T TO BE	
REPR	ODUCED, COPIED OR TRACED IN WH	OLE OR IN PAR	T WITH	TUC
	BOLT ASSEMBLY IN	STALLAT:	ION	
DW		DATE		70
ENG	OH GH	B 0 ET#:	7/05/	/9
LING	TWS	1 OF 1	<u> </u>	
DRA	AWING NO:		RE	V:
	A790135		1	7
	A790133			7